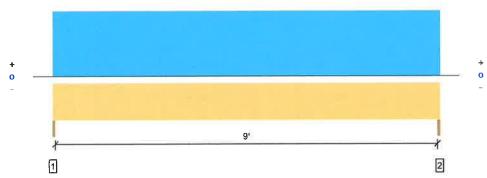
2 piece(s) 1 3/4" x 7 1/4" 2.0E Microllam® LVL

Overall Length: 9' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual © Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	626 @ 0	3806 (1.50")	Passed (16%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	527 @ 8 3/4"	5544	Passed (10%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1448 @ 4' 7 1/2"	8182	Passed (18%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.063 @ 4' 7 1/2"	0.308	Passed (L/999+)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.107 @ 4' 7 1/2"	0.313	Passed (L/999+)		1.0 D + 1.0 S (All Spans)

System: Wall

Member Type : Header Building Use: Residential Bullding Code: IBC 2009 Design Methodology: ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Top Edge Bracing (Lu): Top compression edge must be braced at 9' 3" o/c unless detailed otherwise.
- · Bottom Edge Bracing (Lu): Bottom compression edge must be braced at 9' 3" o/c unless detailed otherwise.

Supports								
	Total	Available	Required	Dead	Roof	Snow	Total	Accessories
1 - Trimmer - SPF	1.50"	1.50"	1.50"	256	370	370	996	None
2 - Trlmmer - SPF	1.50"	1.50"	1.50"	256	370	370	996	None

Loads	Location (Side)	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 9' 3"	N/A	7.4			
1 - Uniform (PSF)	0 to 9' 3"	4'	12.0	20.0	20.0	Residential - Living Areas

Weyerhaeuser Notes

Weyerhaeuser warrants that the sizing of its products will be in accordance with Weyerhaeuser product design criteria and published design values. Weyerhaeuser expressly disclaims any other warranties related to the software. Use of this software is not intended to circumvent the need for a design professional as determined by the authority having jurisdiction. The designer of record, builder or framer is responsible to assure that this calculation is compatible with the overall project. Accessories (Rim Board, Blocking Panels and Squash Blocks) are not designed by this software. Products manufactured at Weyerhaeuser facilities are third-party certified to sustainable forestry standards. Weyerhaeuser Engineered Lumber Products have been evaluated by ICC ES under technical reports ESR-1153 and ESR-1387 and/or tested in accordance with applicable ASTM standards. For current code evaluation reports, Weyerhaeuser product literature and installation details refer to www.weyerhaeuser.com/woodproducts/document-library.

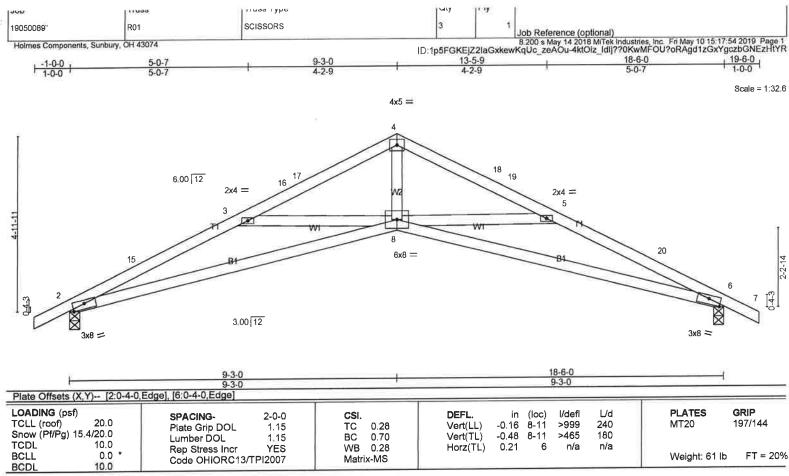
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator



Forte Software Operator	Job Notes	
Thomas Mitchell Elite Home Remodeling (614) 419-5031 twitchell1@msn.com		

					Quote In:		5/10/20	110		Job Numb	er.		19050089QQ	
<u>Estimate</u>					Quote Sent:		5/ 10/20			Customer		:		
Holmes Components - Sunbury					Order Date:			Job Category:			Roof			
3477 N. County Rd. 605					Estimator:		Lyndon	Miller		Sales Rep			Lee Gates	
Sunbury OH 43074 Phone: (740) 936-5077 FAX: (740) 936-5268					Truss Rep:		1			Sch. Deliv				
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Holm	es L	.umi	oer (IR'										
The	Lumberyar	d For Profe	ssionals											
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2598 Ru Columb		13209			Job Notes:									
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											-		7% (7%)	\$30.0
Accepted by Seller					Accepted by Buyer							Grand Total	\$559.8	
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DV:														
BY:					ADDITEGO.									
TITLE:						-								
DATE OF ACCEPT	ANCE:				PHONE:			_ DATE:						

OK JMD



LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=706/0-3-8 (min. 0-1-8), 6=706/0-3-8 (min. 0-1-8)

Max Horz 2=56(LC 12)

Max Uplift2=-68(LC 12), 6=-68(LC 13) Max Grav 2=800(LC 2), 6=800(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-15=-2172/153, 3-15=-2124/172, 3-16=-1641/14, 16-17=-1589/16, 4-17=-1581/30,

4-18=-1581/40, 18-19=-1589/26, 5-19=-1641/23, 5-20=-2124/121, 6-20=-2172/102

BOT CHORD 2-8=-149/1977, 6-8=-66/1977

WEBS 4-8=0/1140, 3-8=-528/189, 5-8=-528/195

NOTES-

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 9-3-0, Exterior(2) 9-3-0 to 12-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow; Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

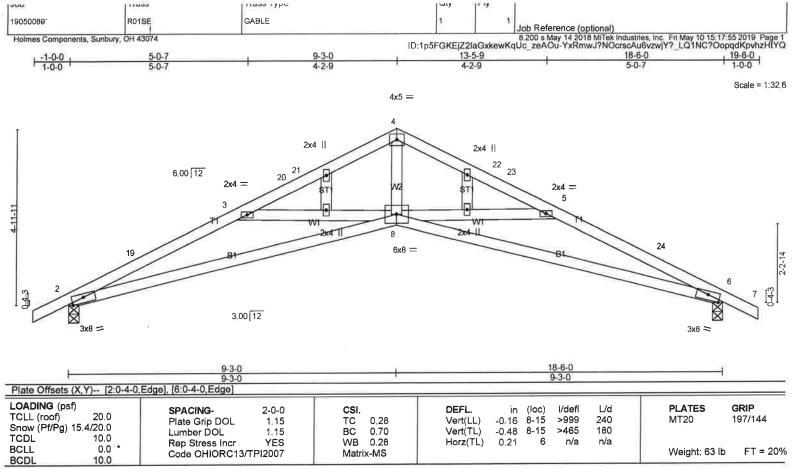
4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

LOAD CASE(S) Standard



LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 OTHERS 2x4 SPF No.2 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-10-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-19=-2172/153, 3-19=-2124/172, 3-20=-1641/14, 20-21=-1589/16, 4-21=-1581/30,

4-22=-1581/40, 22-23=-1589/26, 5-23=-1641/23, 5-24=-2124/121, 6-24=-2172/102

BOT CHORD 2-8=-149/1977, 6-8=-66/1977

WEBS 4-8=0/1140, 3-8=-528/189, 5-8=-528/195

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 9-3-0, Exterior(2) 9-3-0 to 12-3-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-05; Pr=20.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp B; Partially Exp.; Ct=1.1

5) Unbalanced snow loads have been considered for this design.

- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) This truss has been designed for basic load combinations, which include cases with reductions for multiple concurrent live loads.

8) Gable studs spaced at 2-0-0 oc.

- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) Bearing at joint(s) 2, 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.

LOAD CASE(S) Standard



